

CHAPTER 7 PRIMARY RECOMMENDATIONS

Chapter 3 through to Chapter 6 provide detailed recommendations at the end of each chapter for those study components. The reader is referred to each of those chapters for a complete list of recommendations as these are not re-summarized here. This chapter provides the primary recommendations from this study with additional detail provided to assist in any future work. The first recommendation listed, update of the operational guidelines, is a preeminent recommendation as it is required to help ensure the flood hazard for the design event is not greater than that presented in the report and accompanying maps.

7.1 Critical Study Assumption: Modifications to the OLRs Operating Plan

The floodplain maps produced from this study are based on inflows projected to the middle of this century. Magnitudes of the design events are expected to exceed the capacity of the existing infrastructure if operational rules are not adjusted to account for the changing climate. Preliminary modifications to the OLRs Operating plan and guidelines were developed to mitigate projected future increases in floods. If these modifications, or similar mitigations, are not implemented, then the resulting flood flows and levels of the design events are expected to be more severe than mapped.

The preliminary modifications were based solely on flood control. Prior to implementation, any changes to the operational plan is expected to require review initially with the Okanagan Nation Alliance and Canadian Okanagan Basin Technical Working Group (COBTWG) and then with a wider stakeholder group. Given the currently projected rate of change in floods due to climate change, review and subsequent implementation of revised operational rules is recommended within the next five years. The Raven ORB model can be used to explore and optimize different OLRs operation schemes for current and projected conditions. At the time of this future review, any updates to climate projections should be reviewed to assess the most up-to-date expectations on future climate.

The climate projections used for this study followed the most recent, accepted projections available; that is, World Climate Research Programme (WCRP) Coupled Model Intercomparison Project Phase 5 (CMIP5)¹, which was undertaken by the WCRP in support of the IPCC 5th Assessment Report². Climate models have recently been updated with Coupled Model Intercomparison Project Phase 6 (CMIP6)³. Release of these results is scheduled for 2020; the IPCC 6th Assessment Report is due for release in 2022⁴.

7.2 Okanagan Lake Dam and Mainstem Dams

This study did not include an assessment of the consequence of dams overtopping, malfunctioning, dam failure, or the mainstem dams not being operated as designed.

¹ <https://www.wcrp-climate.org/wgcm-cmip/wgcm-cmip5> , accessed 31 March 2020.

² <https://www.ipcc.ch/report/ar5/syr/> , accessed 31 March 2020.

³ <https://www.wcrp-climate.org/wgcm-cmip/wgcm-cmip6> , accessed 31 March 2020.

⁴ <https://www.ipcc.ch/report/sixth-assessment-report-cycle/> , accessed 31 March 2020.

The 2017 freshet resulted in peak lake levels in Kalamalka and Okanagan lakes that exceeded their previously estimated 200-year ARI level (1991 floodplain mapping), and were the highest levels that have occurred since the dams were built (AE, 2017b). Temporary flood barriers (sandbags) on the west side of Okanagan Lake Dam were necessary during the 2017 freshet peak to contain water within the lake and prevent flow around the west side of the main dam and over the (lower) adjacent land (see Photo 1-2). Based on the Lidar data used in this study, the elevation of the adjacent land (~343.12 m CGVD2013) to the west of the dam is lower than the dam crest, and the water level at the time of the 2017 Lidar (343.479 m CGVD2013), which was 0.36 m higher than ground. Preliminary simulation of the maximum flow that could reach the Okanagan River downstream of the dam if the temporary barriers failed, is approximately 275 m³/s. This is more than double the design flood from Okanagan Dam used for the floodplain mapping of this reach (Table 3-32).

The scope of the current study did not include dam or dike breach analysis. Typically dam breach analysis is conducted as part of a dam safety analysis by the dam owner and is not incorporated in floodplain mapping or mitigation analysis. While the simulation of flow around the dam is preliminary and should not be used for any assessment, it highlights a risk that is recommended to be evaluated by the dam owner. The following is a list of dam-related study recommendations:

- Low elevation land to the west and east of Okanagan Lake Dam should be evaluated along with the risk of overflow during flood events. Mitigation measures should be identified and planned for as appropriate.
- Develop formal high-water operating rules and/or emergency plans for each reservoir.
- Risk of dam operations (e.g. blockage or malfunction) that could lead to rapid and/or uncontrolled rise in lake levels, or rapid drawdown and release to the downstream channel should be assessed.
- Infrastructure upgrades should be investigated for each dam with two objectives:
 - Increasing outflow capacity from Okanagan and Kalamalka lakes so that rapidly rising water levels can be responded to more quickly than is currently possible; and,
 - Increasing the capacity of the Okanagan River (e.g. reactivation of side channels, lower floodplain levels, set back dikes) for fish habitat and to increase flow conveyance and also dissipate flood flows. Opportunities that improve fish habitat should be prioritized.
- The current Raven ORB model coupled with hydraulic models of the dams could be used for future Dam Safety reviews, including simulation of the PMF with some model refinement. This can aid in assessing dam breach scenarios, reviewing the mainstem dam consequence classifications (CDA, 2013), and improving emergency response plans.
- As larger flood events become more common and the climate and hydrology of the basin change, it is likely that infrastructure upgrades will become necessary. It should be expected by dam owners that their infrastructure will have to be reassessed and potentially upgraded, possibly multiple times over the coming decades, as climate of the Okanagan region continues to change.

- Further refinement of the rating curves for each dam is recommended to improve reliability in the understanding of discharge through the structures during emergency spills. Limited rating curves, consisting of a one gate open scenario, currently exist for the flow control gates at the outlet of Kalamalka-Wood, Okanagan, Skaha, and Vaseux lakes (WaterSmith Research Inc & Streamworks Consulting Inc, 2014). While total outflow is currently monitored with the use of near real-time discharge data from downstream gauges, this does not allow for such well-informed operations in the case of an outage (during which the real-time data may not be available), which could potentially be needed during an emergency. The real-time gauge data is also reliant on rating curves specifically developed for the river, which may shift and are preliminary until reviewed and published by WSC, with the discharge data reported by the gauge being uncertain during operation.

7.3 Climate Data and Flood Forecasting

The following recommendations are provided for climate monitoring and flow forecasting:

- Automating high-elevation climate stations for which the data is currently manually collected or on a volunteer observer network:
 - It was identified that the ECCC Vernon Silverstar (1128584) is missing most of the critical temperature and precipitation data from the pre-freshet/freshet periods (April-June) for 2016-2019 due to the manual data collection being intermittent. This station's data is being used in the development of climate grids by NRCan¹ and PCIC², and being the only higher elevation EC station in the Okanagan Valley, it influences the interpretation of high elevation precipitation and temperature in the development of the climate grids. As a result of the missing station data, the climate grids for 2016 and 2017 were erroneous (as identified through comparison to SWE data from high elevation snow pillows, which are not directly used in climate grid generation) and could not be used to simulate these freshets with the hydrologic model.
- Include SWE data directly in the generation of future climate grids.
- Installing additional high-elevation climate stations with the number and distribution throughout the ORB based on an assessment of the spatial variability of climate and apparent gaps.
- Improve lower elevation snow monitoring and include this data in flow forecasting models.
- The Raven ORB model could be used for improved water supply forecasting in the basin; however, it is expected that over time, current forecasting methods will be inadequate due to changing peak timing and the approach to forecasting will need to be re-examined in the

¹ <https://cfs.nrcan.gc.ca/projects/3>, accessed 31 March 2020.

² <https://www.pacificclimate.org/>, accessed 31 March 2020.

future. This could involve further exploration into improving long-term forecasting of inflows to the ORB.

- It is recommended that the influence of the Similkameen-Okanogan confluence on Osoyoos Lake levels be simulated with a hydraulic model, and that these results be used to develop a three-dimensional relationship between Similkameen River flow, Okanogan River at Oroville flow, and Osoyoos Lake water level. This relationship could then be used to link a Similkameen hydrologic model to the Raven ORB hydrologic model.

7.4 Mainstem Lakes Data Collection and Further Study

The wave results in this study are limited by the accuracy of the data used. Assessing the impacts of wind and waves on the mainstem lake shorelines could be improved in the following ways:

- Bathymetric surveys (particularly in the nearshore) could improve the accuracy of the nearshore wave results; the available bathymetry for several lakes is particularly coarse, including the narrowing of Osoyoos Lake at Highway 3. This data would improve site specific assessments (but could be collected as such assessment are conducted) in addition it would improve any future revisions to the current study. Site specific assessment is recommended for any locations that appear to vary from the referenced generalized shoreline profile or for sizable developments.
- Implementation of wind and wave buoy data collection on Okanagan Lake. Wave data was not available for this study and was simulated without model calibration; wind data was used from Penticton and Kelowna along with short term wind data collected on the lake in a past study (Spence and Hedstrom, 2015).
- Landslide generated tsunamis have been documented in Okanagan Lake over the last 80 years and can result in runup in excess of that calculated for the wind generated wave events. More detailed study of potential landslide zones, the generation and propagation of tsunamis, and subsequent runup zones should be conducted and added to the floodplain maps. Further work is recommended in assessing tsunami hazard in the Okanagan Valley overall, and that this be provided at minimum as information on floodplain maps or included in FCLs where relevant.

7.5 River and Dikes

The Okanagan River dikes generally contain large floods, but these could fail through stability and seepage mechanisms and can breach even if not overtopped. Where vulnerabilities exist is where the dikes are overtopped and where non-gated culverts are present. The open culverts are a source of floodwater onto the floodplain and can contribute a significant amount of water that can spread a large distance during a long flood. During high flows, particular attention should be paid to the dikes to ensure they are not overtopping, eroding, and that piping through / under the dike is not occurring. If the dike should fail, a large amount of water could access the floodplain. It should be noted that while non-gated

culverts may increase flood risk on the landside of the dikes, they may also act to reduce dike geotechnical failure risk by reducing the differential head across a dike.

Dike assessments and surveys were not completed as part of this study but are recommended for future work. A dike vulnerability assessment would help identify low and weak spots in the dikes and show the available freeboard. Dike crest surveys could be used to update the dike crest heights used in the modelling to support future dike investigations for the Okanagan Basin.

7.6 Flood Event Monitoring

HWMs should be collected during all flood flow events (i.e. floods in excess of the 10-year flood). This information is useful for model calibration and validation, which should be done following any large flood event. The HWM's should be taken along the entire profile of the modelled reach with sufficient spacing to capture any substantial breaks in slope (i.e. such as upstream and downstream of all hydraulic structures and any constrictions or change in slope). The HWM's for a particular flood event should be collected at the same flow, ideally near the peak of the flood event. The HWM can be staked during the flood and surveyed after it recedes as long as the duration is not so long following the flood that the HWMs maybe damaged or moved.

7.7 Floodplain Mapping, Applications, and Website

Recommendations for future work include:

- Keep the study and website live and up-to-date and continue to share the information on flood hazard with local communities through in-person workshops and discussions to ensure the information is available and accessible. These workshops help the community through dissemination of information about the hazard, identifying what matters to the community and stakeholders, educating people on the issues and options, as well as identifying and building momentum to proceed with further flood risk reduction measures.
- Develop a comprehensive risk reduction plan. This plan should take a risk-based approach to flood mitigation, include stakeholder priorities, be developed for the short, medium, and long term, consider climate change, and strive for consistency between jurisdictions. Key steps in plan development are:
 - Assessment of existing structural and non-structural flood mitigation measures;
 - Assessment of flood risk to identify areas of high risk and in greatest need of further flood mitigation, and to inform prioritization of mitigation measures; and,
 - Identification of structural and non-structural risk mitigation measures and development of an actionable plan to implement the measures.

Re-evaluate and maintain studies associated with the plan including floodplain mapping, risk assessment, and mitigation planning to reflect changes in risk related to hazard likelihood or consequence. Flood hazard maps should be reviewed at least every 10 years and updated if there are

changes to conditions such as the design flood, channel geometry, new flood hazards, dike construction, or floodplain development, as recommended by EGBC (APEGBC, 2017).

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